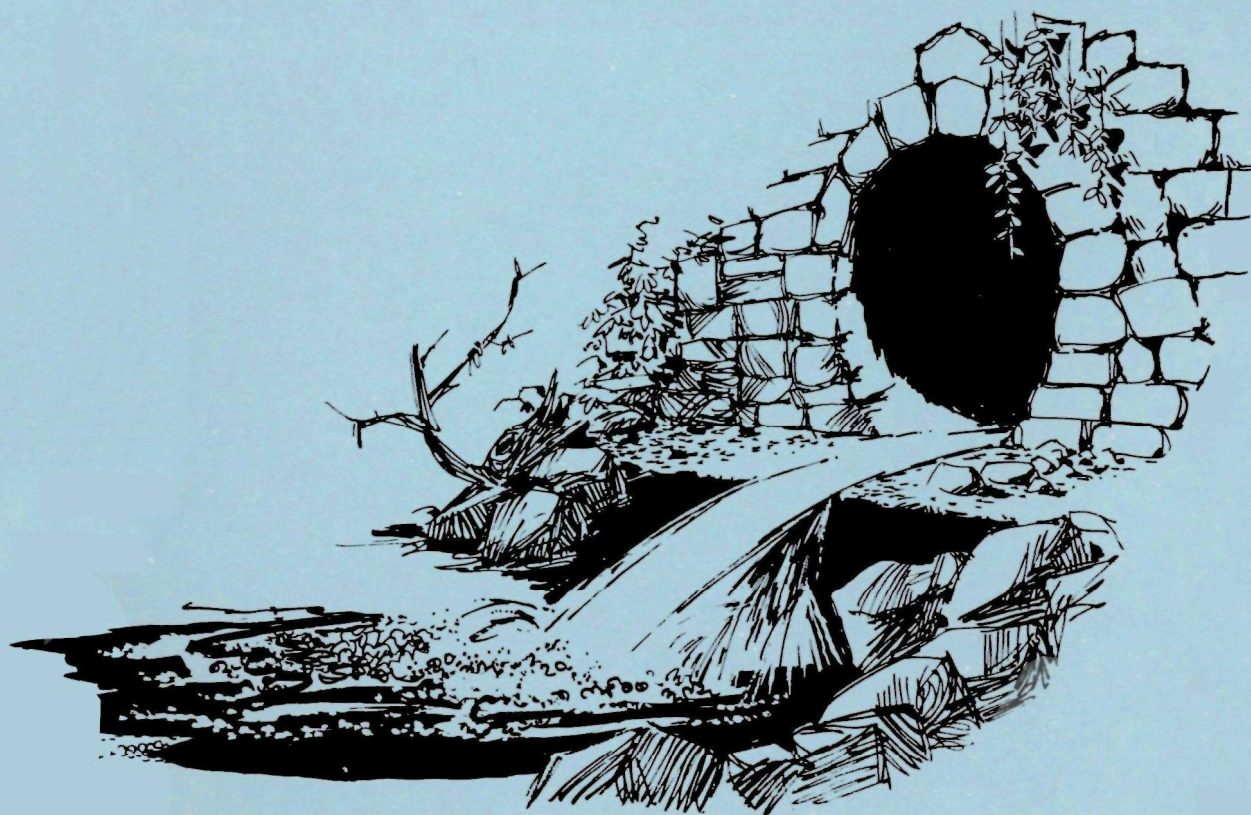


# **Demonstration of Rotary Screening For Combined Sewer Overflows**



### WATER POLLUTION CONTROL RESEARCH SERIES

The Water Pollution Control Research Series describes the results and progress in the control and abatement of pollution in our Nation's waters. They provide a central source of information on the research, development, and demonstration activities in the Water Quality Office, Environmental Protection Agency, through inhouse research and grants and contracts with Federal, State, and local agencies, research institutions, and industrial organizations.

Inquiries pertaining to Water Pollution Control Research Reports should be directed to the Head, Project Reports System, Office of Research and Monitoring, Environmental Protection Agency, Room 801, Washington, D.C. 20242.

DEMONSTRATION OF ROTARY SCREENING  
FOR  
COMBINED SEWER OVERFLOWS

For  
ENVIRONMENTAL PROTECTION AGENCY

By  
CITY OF PORTLAND, OREGON  
DEPARTMENT OF PUBLIC WORKS  
BUREAU OF SANITARY ENGINEERING

Program No.-11023 FDD  
Contract 14-12-128 Modification No. 7  
July, 1971

## EPA Review Notice

This report has been reviewed by the Water Quality Office and approved for publication. Approval does not signify that the contents necessarily reflect the views and policies of the Environmental Protection Agency, nor does mention of trade names or commercial products constitute endorsement or recommendation for use.

## ABSTRACT

The objective of this demonstration was to determine screen durability, solids removal, COD removal, and hydraulic efficiency of rotary fine screening of storm-caused combined sewer overflows.

2300 gpm were evenly distributed to a 60 inch diameter rotating (55 rpm) screen cage holding 18 ft<sup>2</sup> of 165 mesh stainless steel screens (105 micron opening, 47.1 percent open area). During a screening cycle a concentrate sensor stopped the sewage pumps, ending the screening phase and initiating a 30 second cleaning phase during which the screens were automatically washed. At the end of the cleaning phase the pumps restarted automatically and a new cycle began.

Performance on storm-caused combined sewage flow averaged 54.8 percent removal of settleable solids, 26.6 percent removal of suspended solids, and 15.5 percent removal of COD. Duration of the screening phases averaged 14.6 minutes with average hydraulic efficiencies dropping from 0.880 to 0.668.

The ultimate screen life varied from a minimum of 190.5 hours to a maximum of 516 hours with an average of 346. Screens required an average of 3.5 repairs during this life. (Schmidt - Portland).

# TABLE OF CONTENTS

SECTION	TITLE	PAGE
	ABSTRACT - - - - -	iii
	TABLES - - - - -	vii
	FIGURES - - - - -	ix
1	CONCLUSIONS - - - - -	1
2	RECOMMENDATIONS - - - - -	3
3	INTRODUCTION - - - - -	5
	Combined Sewage-Stormwater Overflow - - -	5
4	DEMONSTRATION PROCEDURE - - - - -	7
	Site Description - - - - -	7
	Screening Plant Layout - - - - -	7
	Description and Operation of Screening Equipment - - - - -	7
	Sampling Program - - - - -	11
	Observations - - - - -	11
5	MODE OF OPERATION - - - - -	13
6	DISCUSSION OF RESULTS - - - - -	15
7	ACKNOWLEDGMENTS - - - - -	27
8	GLOSSARY - - - - -	35
9	APPENDICES - - - - -	39
	Screen Cleaning Agents - - - - -	40
	Data Presentation - - - - -	41
	Supplemental Data - - - - -	52
	Detailed Methods of Analysis - - - - -	53
	Screen Life - High Velocity - - - - -	54
	Screen Life - Low Velocity - - - - -	55

# TABLES

NUMBER	TITLE	PAGE
1	DETAILED SAMPLING PROGRAM - - - - -	12
2	HYDRAULIC EFFICIENCY - - - - -	16
3	SCREEN LIFE - - - - -	17
4	REMOVAL RATES/RAIN CAUSED FLOW (3 MGD)	30
5	REMOVAL RATES/DRY WEATHER FLOW (3 MGD)	31
6	REMOVAL RATES/RAIN CAUSED FLOW (2.5 MGD)	32
7	REMOVAL RATES/DRY WEATHER FLOW (2.5 MGD)	33
8	REMOVAL RATES/DRY WEATHER FLOW HIGH VELOCITY (3 MGD) - - - - -	34

# FIGURES

NUMBER	TITLE	PAGE
1	GENERAL LAYOUT, SULLIVAN GULCH PUMP STATION AND SCREENING FACILITY - - - - -	9
2	SAMPLING DIAGRAM - - - - -	18
3	SCREENING UNIT - - - - -	19
4	SCREENING CYCLE - - - - -	20
5	RAINFALL PORTLAND, OREGON - 1970 - - - - -	21
6	FLOW CHART - - - - -	22
7	FLOW CHART - - - - -	23
8	IMHOFF CONE STREAM FLOW COMPARISONS - - - -	24
9	TYPICAL SCREEN FAILURES - - - - -	25



## SECTION 1

### CONCLUSIONS

1. Effective screening of storm caused combined sewage overflow is reduced substantially in the presence of oil and grease. Frequent backwashing is required to return the screens to their original capacity.
2. Paint has a detrimental effect on hydraulic efficiency and only after hand application of concentrated cleaner, Zep 9658, could the screens be returned to normal efficiency.
3. Screen failures were attributable to two causes, physical breakdown and puncture.
4. At low velocity an average screen was repaired 3.5 times before ultimate failure. At high velocity an average screen was repaired 2.3 times before ultimate failure.
5. Alkaline, acidic, and alcoholic agents did not adequately clean the screens. Chloroform, solvent parts cleaner, soluble pine oil, Zif, Formula 409, and Vestal Eight offered limited effectiveness. Zep 9658 cleaned the screens effectively but it should be noted that water quality implications were not determined.
6. Appreciable quantities of frothy floating oil were noted in the relatively quiescent baffled trough which served as an effluent channel.

## SECTION 2

### RECOMMENDATIONS

1. It is recommended a study be made to determine hydraulic efficiency of the screens at pre-determined grease loadings. Such a study would facilitate predicting hydraulic performance of screening equipment when grease loading of the sewer is known.
2. It is recommended a study be made for finding an economical screen cleaning agent, one which would be environmentally acceptable when discharged into a receiving water.
3. It is recommended a study be made to find a practicable method to skim the oil from the screened effluent in the effluent channel where appreciable quantities of frothy floating oil were observed passing over the weir into the receiving water.

## SECTION 3

### INTRODUCTION

#### COMBINED SEWAGE STORMWATER OVERFLOW

The majority of the existing sewers in the City of Portland, Oregon, carry combined storm and sanitary flows. Only three times average dry weather flow reaches the sewage treatment plant. The storm caused flow, above three times average flow, bypasses to the receiving streams thereby causing pollution of the water course. To correct this condition, separate storm and sanitary sewers would be required.

A method proposed to reduce the cost of separate sewers is the installation of high-rate, fine mesh screening units on outfall sewers to intercept and provide primary treatment of the storm overflow. The feasibility of this method has been researched at this location prior to this study. It was the objective of this test period to determine ultimate screen life, solids removal efficiency, C.O.D. removal efficiency and hydraulic efficiency of the SWECO screening equipment at the rate of 3.3 MGD during storm flow conditions.

For storm water testing of the screenings unit, it was assumed that a 50 percent increase of the recorded average daily flow at the Sullivan Sewage Pumping Station, would be considered storm caused flow.

Monthly rainfall records for the period of operation are shown in figure No. 5.

## DEMONSTRATION PROCEDURE

## SITE DESCRIPTION

The screening facility is located adjacent to the Sullivan Sewage Pumping Station in Portland, Oregon. The Sullivan Station serves a drainage basin of approximately 25,000 acres of Portland's metropolitan area from which it pumps up to 53 (MGD) million gallons a day. This area is mainly residential with approximately 30,000 single-family residences within its boundaries. The usual services are available within this area to support the population. Paint and automobile related industries are well represented in this area as indicated by their waste products frequently visible in the screened effluent channel. Since these are combined sewers, the usual undesirable amounts of oils and fats are present in varying amounts. During prolonged rainy periods, these amounts are, understandably, less concentrated. The 72-inch interceptor sewer has a capacity upwards of 59 MGD. The Sullivan Pumping Station is adequately sized to handle the flow without by-passing.

The flow to the screening facility was pumped from the Sullivan Pump Station by-pass channel at all levels of flow into the station.

## SCREENING PLANT LAYOUT

Figure 1 illustrates the layout of the screening facility adjacent to the Sullivan Sewage Pumping Station. Combined sewage flows to the Sullivan Station through a 72-inch horseshoe interceptor sewer. The flow enters the by-pass chamber through a coarse bar screen before reaching the screening facility pumps. The two vertical turbine pumps are capable of lifting combined sewage flow at the rate of 3.6 MGD.

In a typical installation on an outfall sewer, the flow passing through the screens would pass to a receiving stream after disinfection. The retained flow would be returned to an interceptor sewer. In this demonstration installation, both flows are returned to the Sullivan Pumping Station.

## DESCRIPTION AND OPERATION OF SCREENING EQUIPMENT

Figures 3 and 4 show sections of the SWECO screening unit. This unit stands 69 inches high with an outside diameter of 84 inches. The flow enters near the bottom of the unit and passes up through a pipe in the center of the unit onto a horizontal distribution dome. This flighted dome spreads and directs the flow downward against the inner top surface of the screens. The manufacturer reports an impingement velocity

of approximately 15 feet per second at the screen. This flow action together with the centrifugal force resulting from the rotation of the screen cage and the characteristics of the influent determine the percentage of the flow through the screen and that concentrate flow retained inside the screen. The unit is equipped with a cleaning device which activates when the concentrate flow reaches a predetermined level. Hot water (170 degrees F) from a commercial water heater and tar and asphalt remover, Zep 9658, were used to clean the screens. The cleaner was injected into the hotwater piping with a positive displacement pump at dilutions varying with the consistency of the sewage. During the cleaning phase, the pump stops and the screen cage continues to revolve at 55 rpm while spray nozzles, located outside the screens, blast solids and grease back into the concentrate bowl, then the inside nozzles operate with each set alternating twice during the 30 second cleaning phase. This cleaning returns the screens to their initial hydraulic capacity except when materials like paints and heavy asphalts are present. When such materials are present, the screens must be cleaned manually with concentrated cleaner. The materials which cannot pass through the 105 micron screens are contained and drop down inside of the screens to a concentrate bowl located below the screen cage and are discharged by gravity.

The screened effluent is collected in a concentric annular chamber box at the bottom of the unit. The screen cage drive is located at the top of the screening unit and is driven at 55 rpm by a 5 HP induction motor.

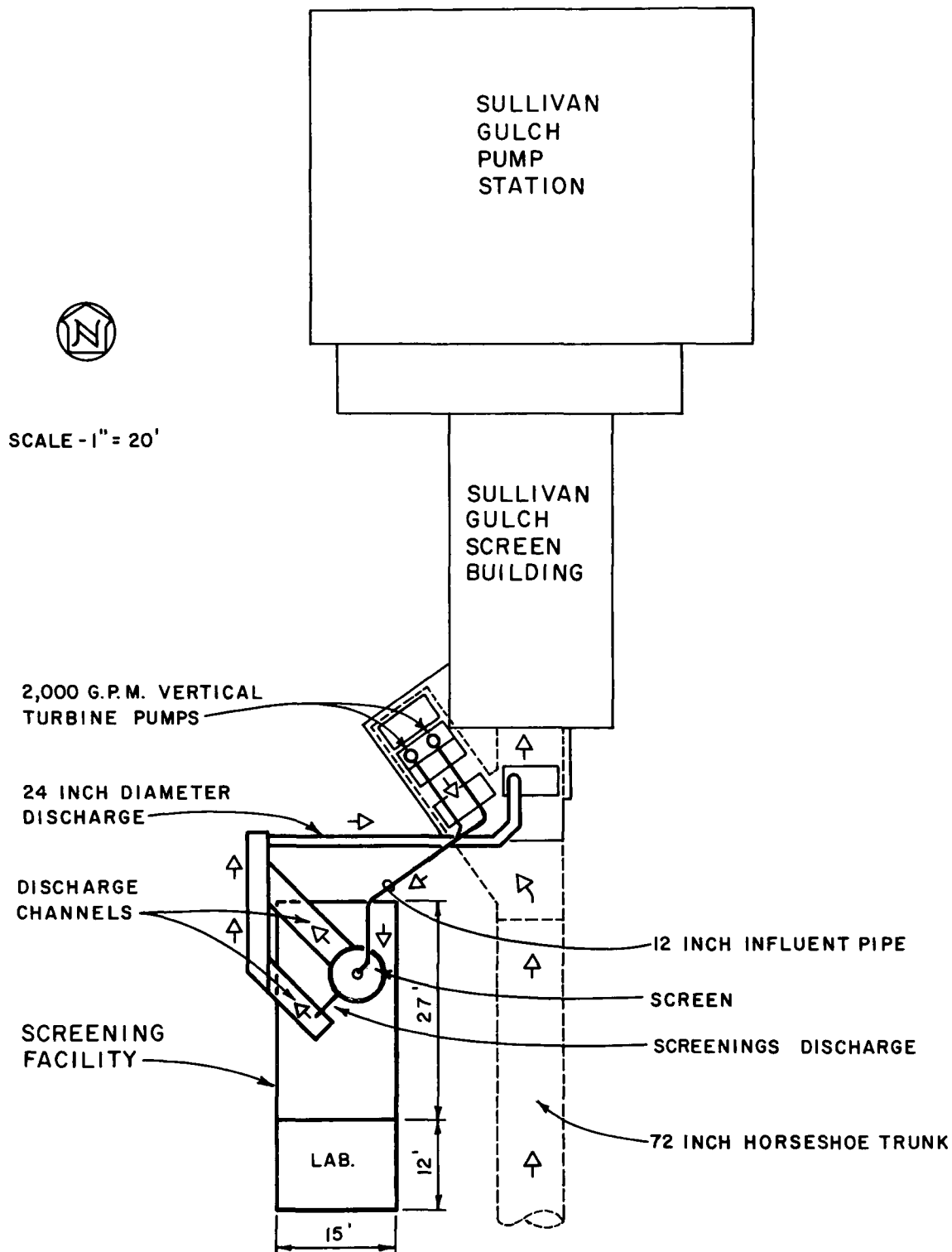


FIGURE 1  
GENERAL LAYOUT  
SULLIVAN GULCH PUMP STATION  
AND SCREENING FACILITY  
PORTLAND, OREGON

## SAMPLING PROGRAM

Three grab samples of equal volume were taken during a one hour time period at twenty minute intervals to make up a one hour composite sample. The influent samples were taken from a one inch line attached to the influent supply pipe. The effluent samples were taken at the sampling point provided for on the screening unit. Table 1 is a summary of the sampling program.

A schematic diagram of the screening facility, the process streams and the observations made on each stream are shown on figure 2.

OBSERVATIONS	PROCESS STREAM	SAMPLE FREQUENCY <sup>1</sup>	ANALYTICAL METHOD
SETTLEABLE SOLIDS	Influent and screened effluent	twice/day	Appendix page 83
SUSPENDED SOLIDS	Influent and screened effluent	twice/day	Wyckoff, "Rapid Solids Determination using Glass Fiber Filters", <u>Water and Sewage Works</u> , June 1964.
COD	Influent and screened effluent	twice/day	Jeris, "A Rapid COD Test" <u>Water and Wastes Engineering</u> , May 1967.
FLOW RATE	Screened effluent and solids concentrate	continuously	recorder
BACKWASH FREQUENCY	- - -	continuously	recorder
SCREEN LIFE	- - -	- - -	Visual observation and log
BOD	Influent and screened effluent	once/week	<u>Standard Methods</u> , 12th Ed.
TOTAL SOLIDS	Influent, effluent, concentrate	once/week	" " " "
VOLATILE SOLIDS	Influent, effluent, concentrate	once/week	" " " "
CHLORINE REQUIREMENT	Influent, effluent	twice/day	Appendix page 83
GREASE	Influent, effluent, concentrate	once/week	Appendix page 83

1- A sample is a one-hour composite consisting of three grab samples

TABLE 1  
DETAILED SAMPLING PROGRAM



## SECTION 5

### MODE OF OPERATION

During the testing program the machine was in operation under test conditions for a total of 914 hours. The first 76 hours the screen cage was rotated at 65 rpm. The 3 MGD flow was fed to the unit at high velocity (the velocity was not determined). The high velocity was accomplished by placing an extension piece on the inlet pipe and tightening down on a movable deflection plate thereby decreasing the flow area.

It was discovered that general screen life was not acceptable under these operating conditions; therefore, the screen cage was reduced to 55 rpm, the extension piece and deflection plate were removed and the flow reduced to 2.5 MGD. These low velocity conditions were maintained for a total of 188 hours. During the remaining 650 hours, the machine was operated at 3 MGD and the screen was rotated at 55 rpm without the extension piece and high velocity plate attached.

It should be understood that process stream flows are approximations only and may range as follows: 3 MGD (high velocity) varied between 3.1 MGD and 3.8 MGD, 3 MGD (low velocity) varied between 3.2 MGD and 3.6 MGD, 2.5 MGD (low velocity) varied between 2.5 MGD and 3.3 MGD. Actual flows are dependent on the length of the screening cycle.

During the testing program, the screens were inspected about once every two hours for failures. This was done by shutting the machine off and observing the screens through the window or the opening provided in the top of the machine while turning the screen cage by hand. Shining a light on the screens made any holes or rips quite visible. If a screen developed a large rip (4" or larger) it could usually be detected by the change it caused in the flow pattern on the window while the machine was running.

Screens were patched with epoxy glue; and after mastering the repair technique, the screens can be back in operation within an hour after they have been removed.

During the 914 hours of operation, the main bearing required lubrication three times. This was accomplished by lifting the hinged motor cover to expose the grease fitting and could be done in five minutes.

°

## SECTION 6

### DISCUSSION OF RESULTS

The results of the EPA Research and Development Project ("Rotary Vibratory Fine Screening of Combined Sewer Overflows", Program 11023 FDD) by Cornell, Howland, Hayes and Merryfield, indicated that screen life was quite short and that testing under this contract should include screen durability tests under rain caused combined sewer overflows.

Preliminary to the final testing program, the screen rotation was set at 65 RPM and the sewage inflow pipe below the distribution pan, was extended for the purpose of increasing flow velocity. It was hoped that this increased velocity would increase hydraulic performance.

The combination of the 65 RPM screen speed and the high velocity sewage flow proved to be disastrous to general screen life.

The screen velocity was reduced to 55 RPM and the extension fitting on the inflow pipe was removed. The flow was also reduced to approximately 2.5 MGD. After considerable testing it became apparent that the high velocity of the screen and the increased velocity through the extension fitting were primarily responsible for the short screen life and that the flow could again be increased to 3 MGD without detriment.

Final testing of the screening unit was conducted at the lower velocity inflow at 3 MGD with a screen speed of 55 rpm.

The results of the screen life tests are shown on Table No. 3.

OPERATING CONDITIONS AND NUMBER OF OBSERVATIONS	HYDRAULIC EFFICIENCY <sup>1</sup> AT START OF SCREENING PHASE			HYDRAULIC EFFICIENCY AT END OF SCREENING PHASE			SCREENING PHASE LENGTH IN MINUTES		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
3 MGD DRY 20 OBSERVATIONS HIGH VELOCITY <sup>2</sup>	0.848	0.892	0.805	0.768	0.811	0.733	8.0	2.8	25.0
2.5 MGD DRY 31 OBSERVATIONS LOW VELOCITY <sup>2</sup>	0.833	0.873	0.764	0.659	0.773	0.570	9.6	4.5	25.0
2.5 MGD RAIN 6 OBSERVATIONS LOW VELOCITY	0.822	0.843	0.804	0.670	0.688	0.617	9.6	5.0	20.0
3 MGD DRY 20 OBSERVATIONS LOW VELOCITY	0.878	0.915	0.840	0.750	0.900	0.656	11.2	4.0	40.0
3 MGD RAIN 26 OBSERVATIONS LOW VELOCITY	0.880	0.920	0.837	0.768	0.894	0.650	14.6	4.0	56.0

1- SEE GLOSSARY

TABLE 2  
HYDRAULIC EFFICIENCY

2- SEE GLOSSARY

FLOW AND OPERATING CONDITIONS	INITIAL SCREEN LIFE (HRS.)			NUMBER OF REPAIRS			ULTIMATE SCREEN LIFE (HRS.) 1		
	Ave.	Min.	Max.	Ave.	Min.	Max.	Ave.	Min.	Max.
3.1 - 3.8 MGD FLOW HIGH VELOCITY 65 RPM 18 SCREENS TESTED	20.5	1.8	35.5	2.3	0	7	34.3	16.5	95.3
2.5 - 3.6 MGD FLOW LOW VELOCITY 55 RPM 17 SCREENS TESTED	233.2	32	336.8	3.5	0	10	346.	190.5	516.

1- The ultimate screen life as reported above is actual running hours of the screens, and the life of an individual screen ended only when it could no longer be successfully repaired. See Appendix .

TABLE 3  
SCREEN LIFE

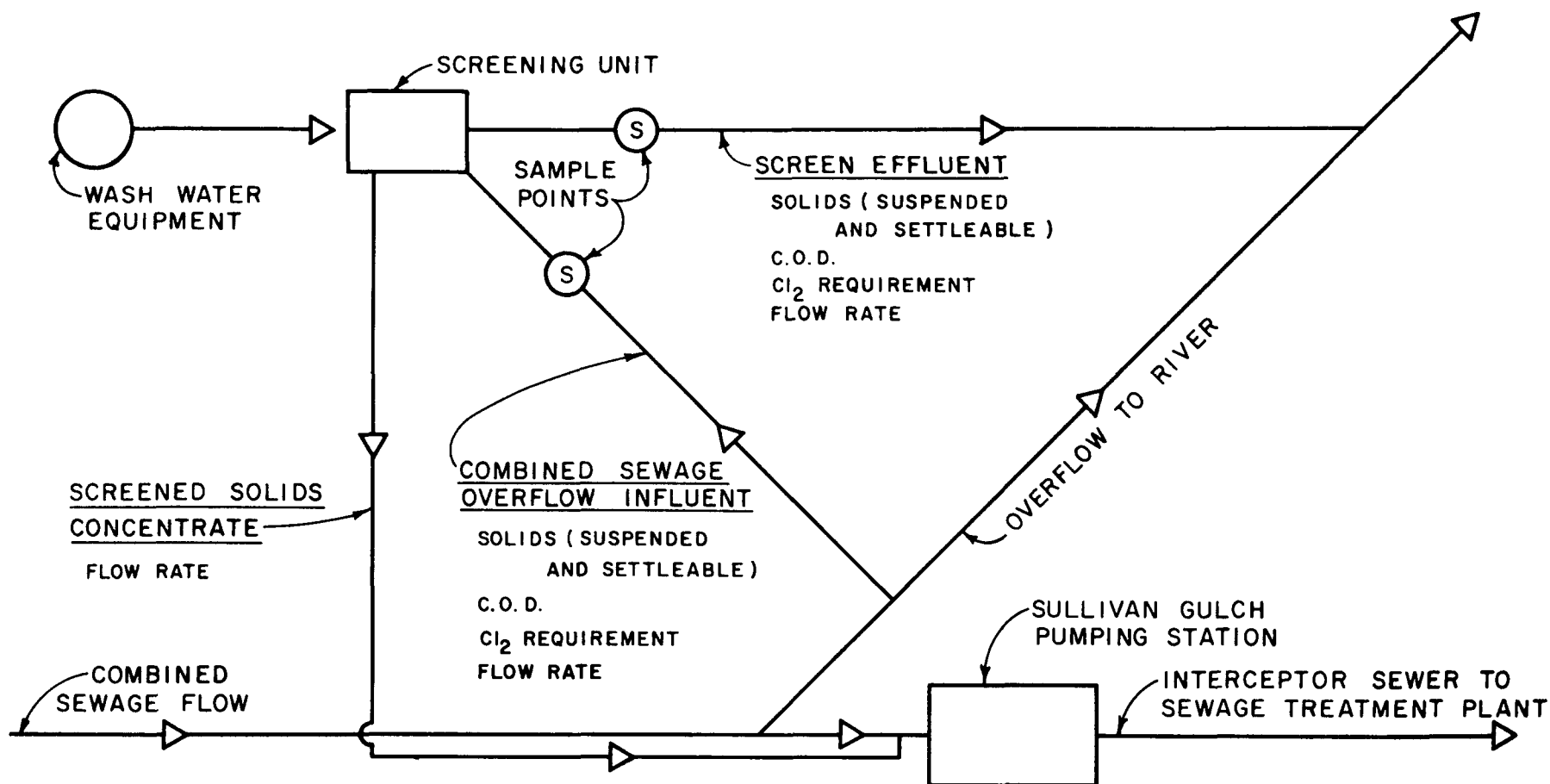


FIGURE 2  
TYPICAL  
COMBINED SEWAGE OVERFLOW SCREENING  
SAMPLING DIAGRAM

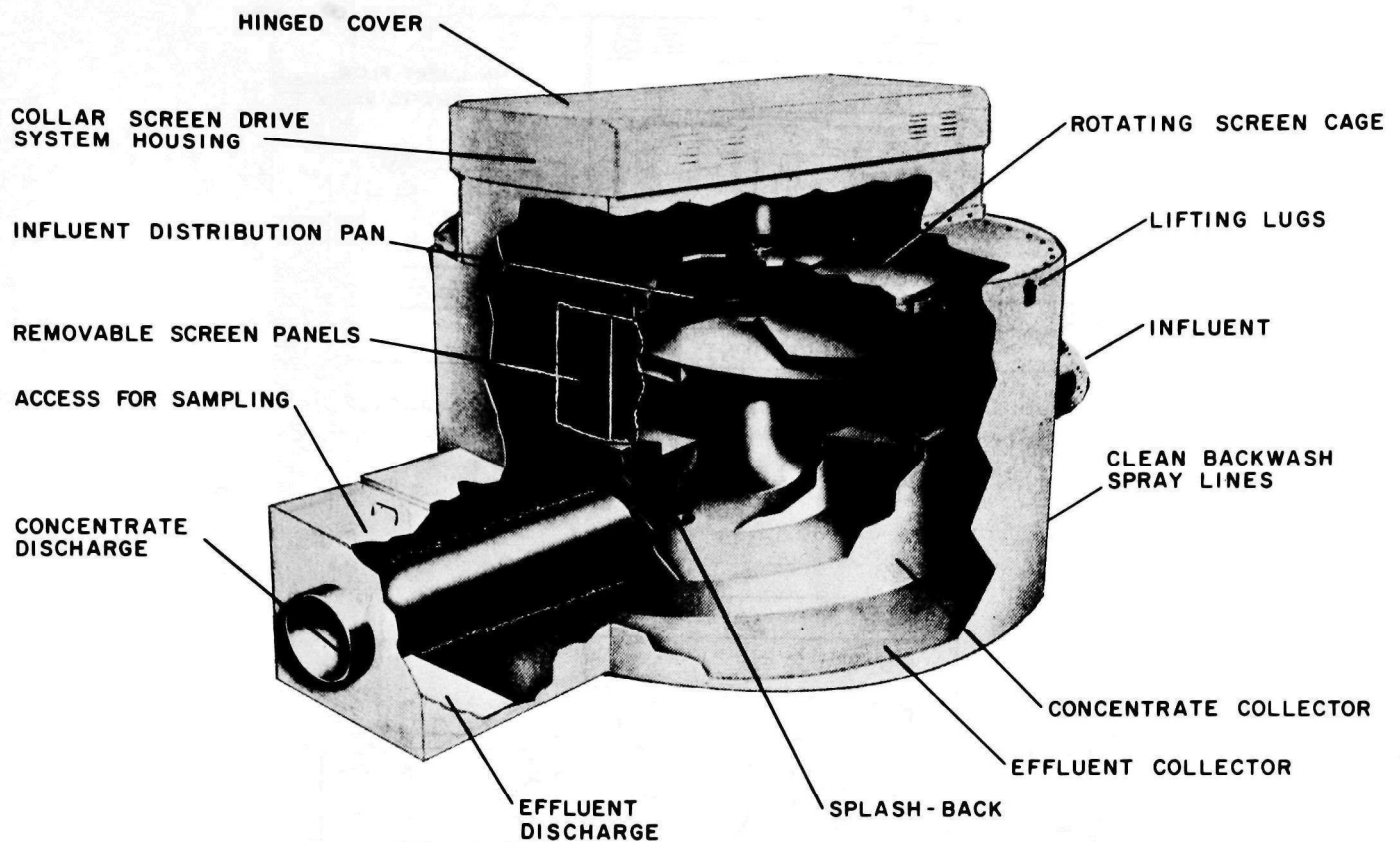
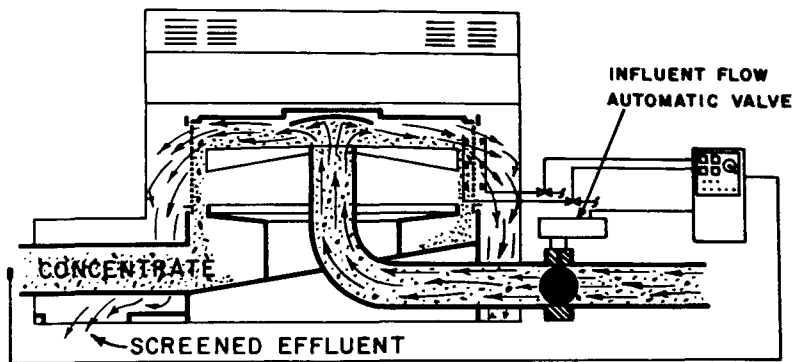
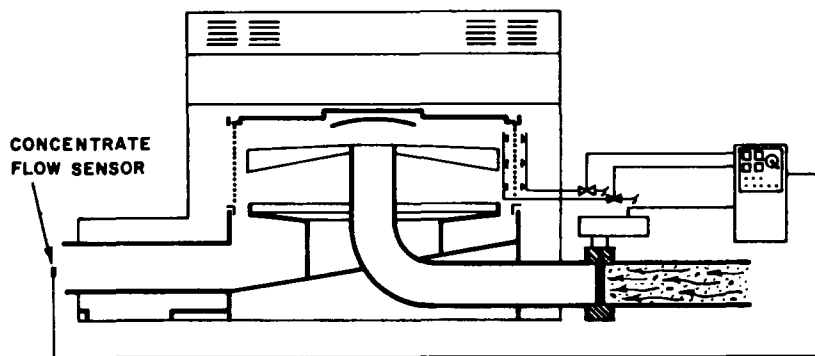


FIGURE 3  
SWECO WASTEWATER CONCENTRATOR

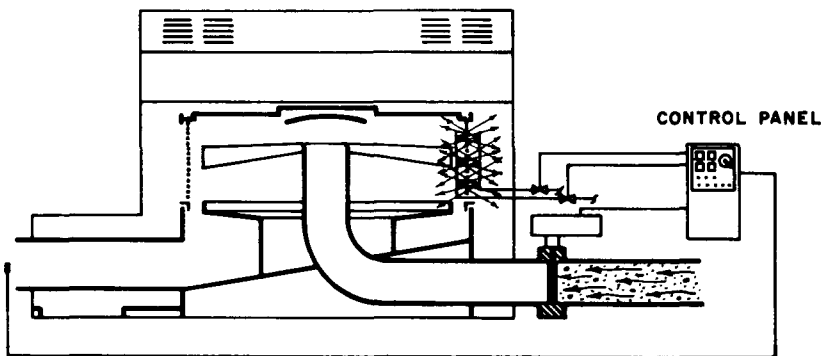
This unit stands 69 inches high with an outside diameter of 84 inches. The flow enters near the bottom of the unit and passes up through a pipe in the center of the unit onto a horizontal distribution dome. This flighted dome spreads and directs the flow downward against the inner top surface of the screens. The manufacturer reports an impingement velocity of approximately 15 feet per second at the screen. This flow action together with the centrifugal force resulting from the rotation of the screen cage and the characteristics of the influent determine the percentage of the flow through the screen and that concentrate flow retained inside the screen.



Influent flow valve open: flow continues until the pre-determined maximum concentrate level is reached. (Monitored by Sensor)

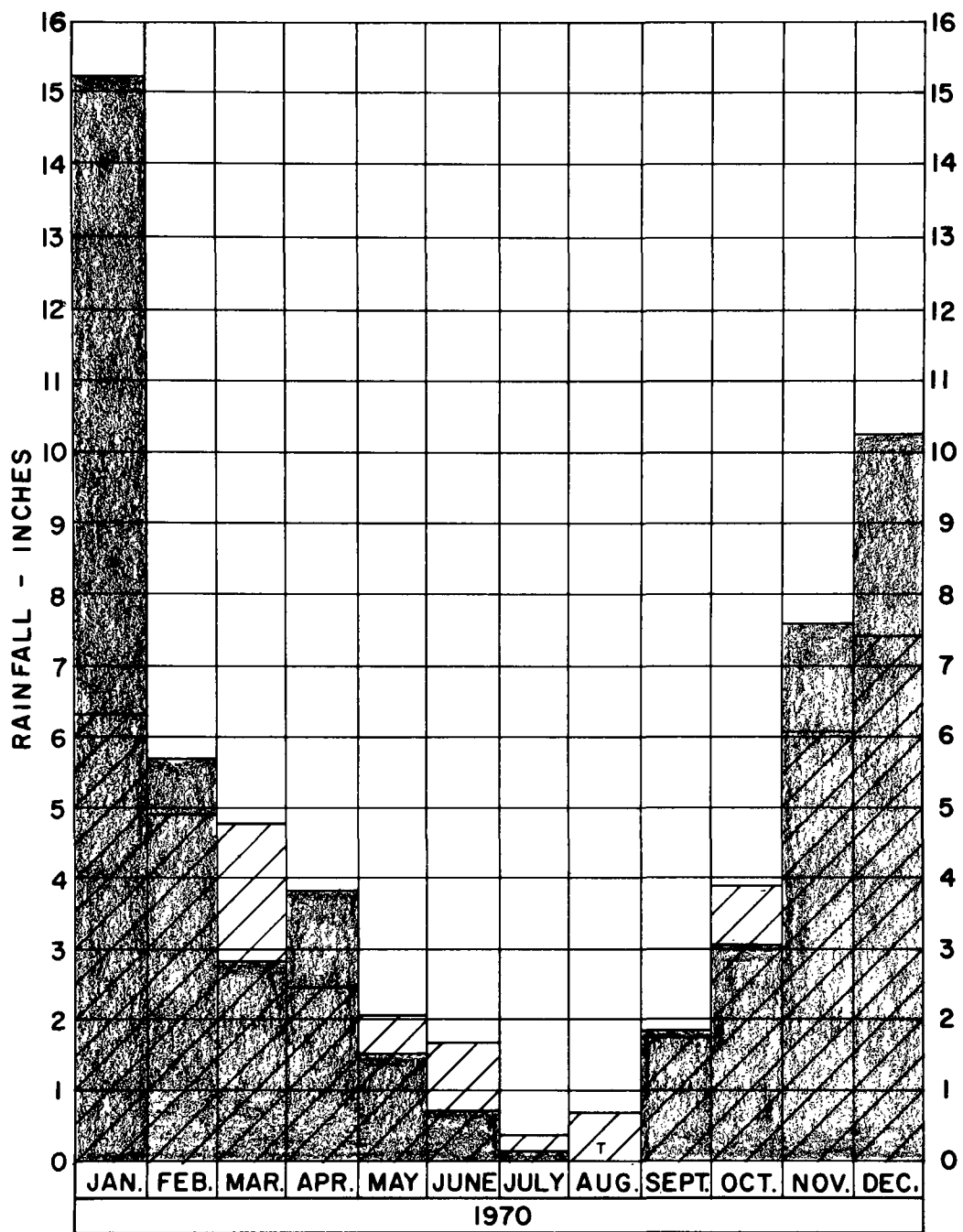


Influent flow valve closes: Concentrate flow has reached pre-determined maximum level. Cleaning phase is then initiated.



Cleaning phase: The screen cage continues to revolve during the 30-second cleaning phase. First the outside nozzles spray, then the inside, with each set operating twice during the phase. Following the cleaning, the influent flow valve automatically opens and the screening cycle is repeated.

**FIGURE 4  
SCREENING CYCLE**



 MULTI-YEAR MONTHLY AVERAGE
  MONTHLY TOTAL

FIGURE 5  
 RAINFALL  
 PORTLAND, OREGON



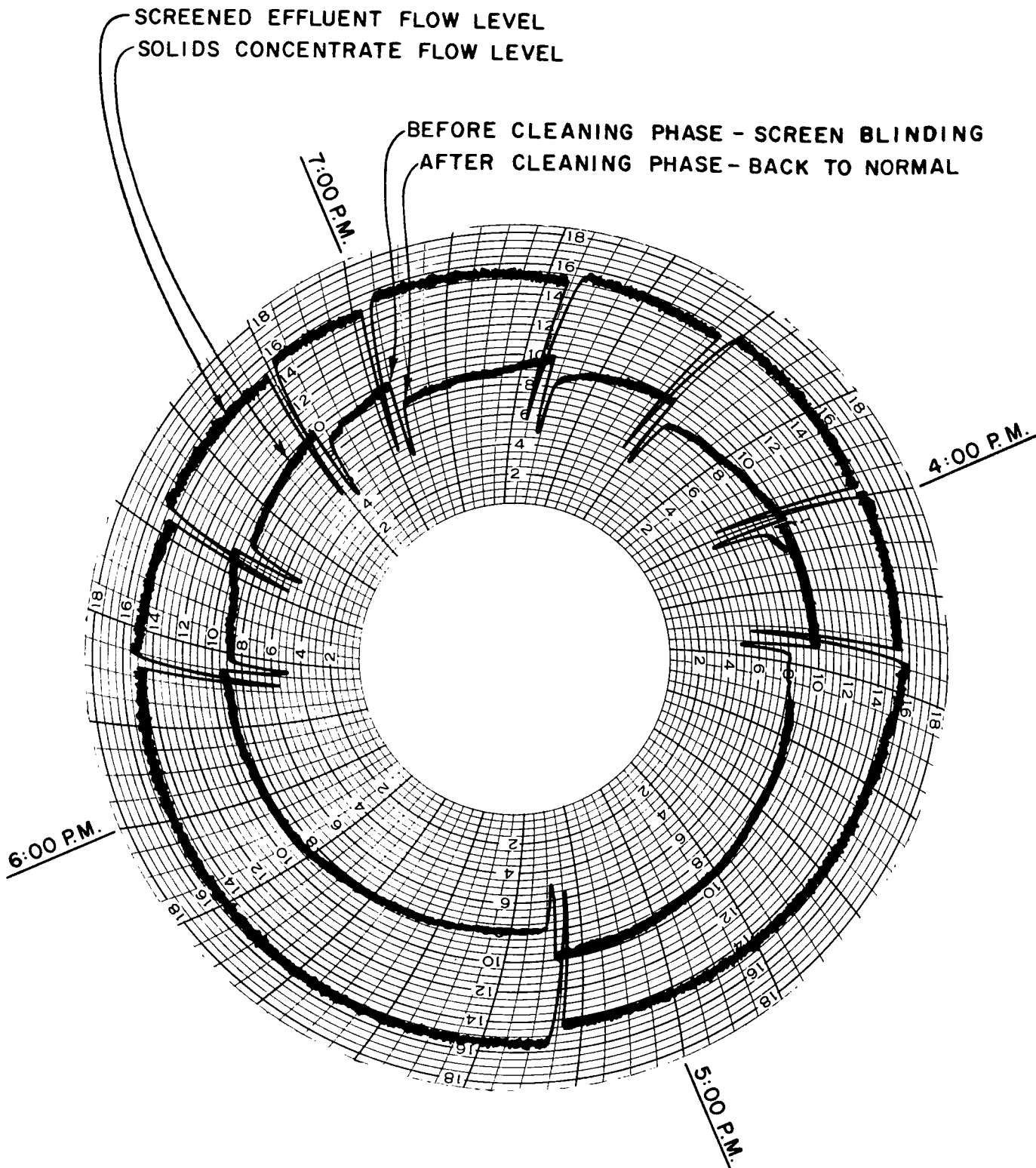


FIGURE 6  
INDICATION OF  
LOW SOLIDS AND LOW GREASE LOADINGS  
OVER FOUR-HOUR PERIOD

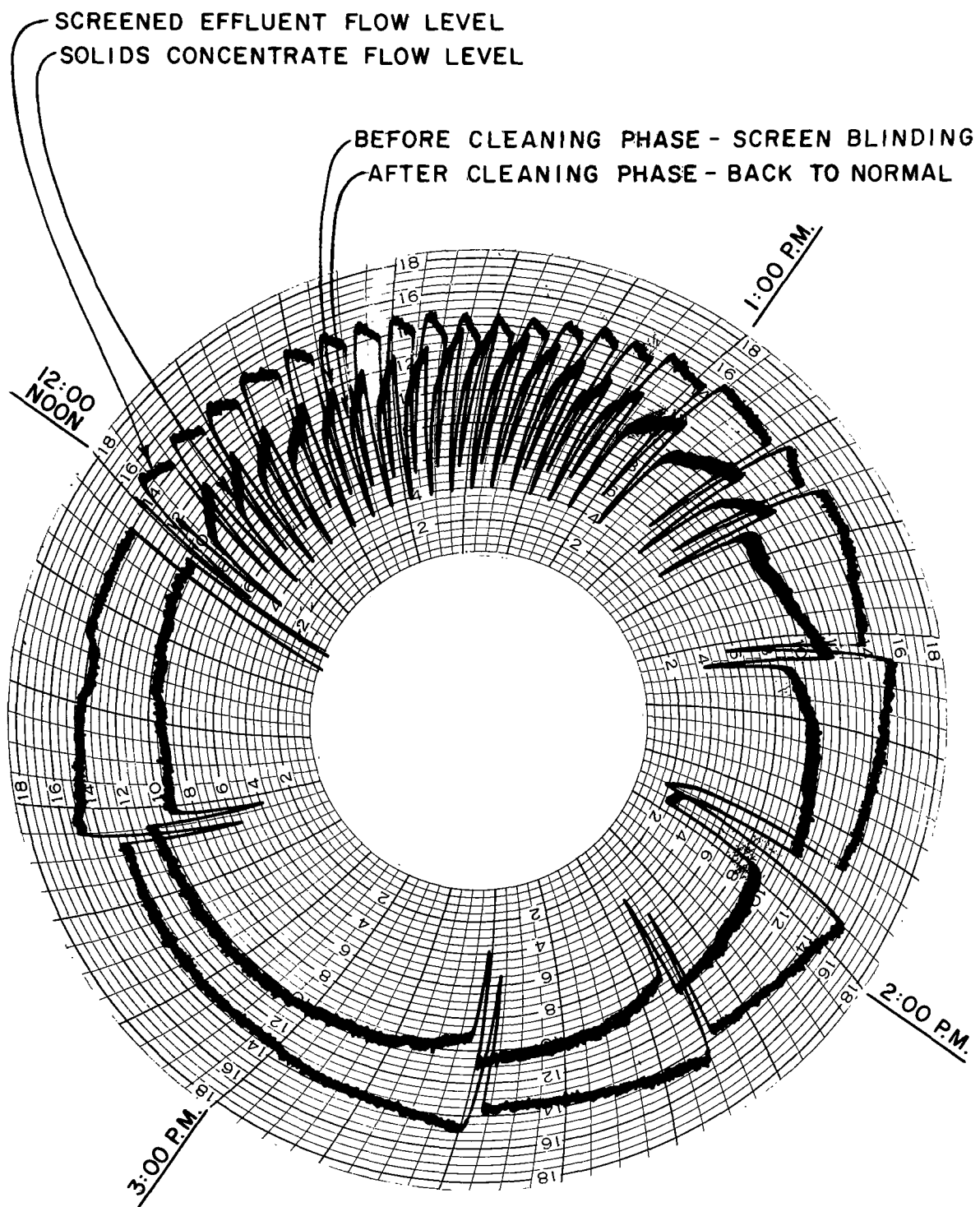


FIGURE 7  
INDICATION OF  
SEWAGE CHARACTERISTIC CHANGES  
OVER FOUR-HOUR PERIOD

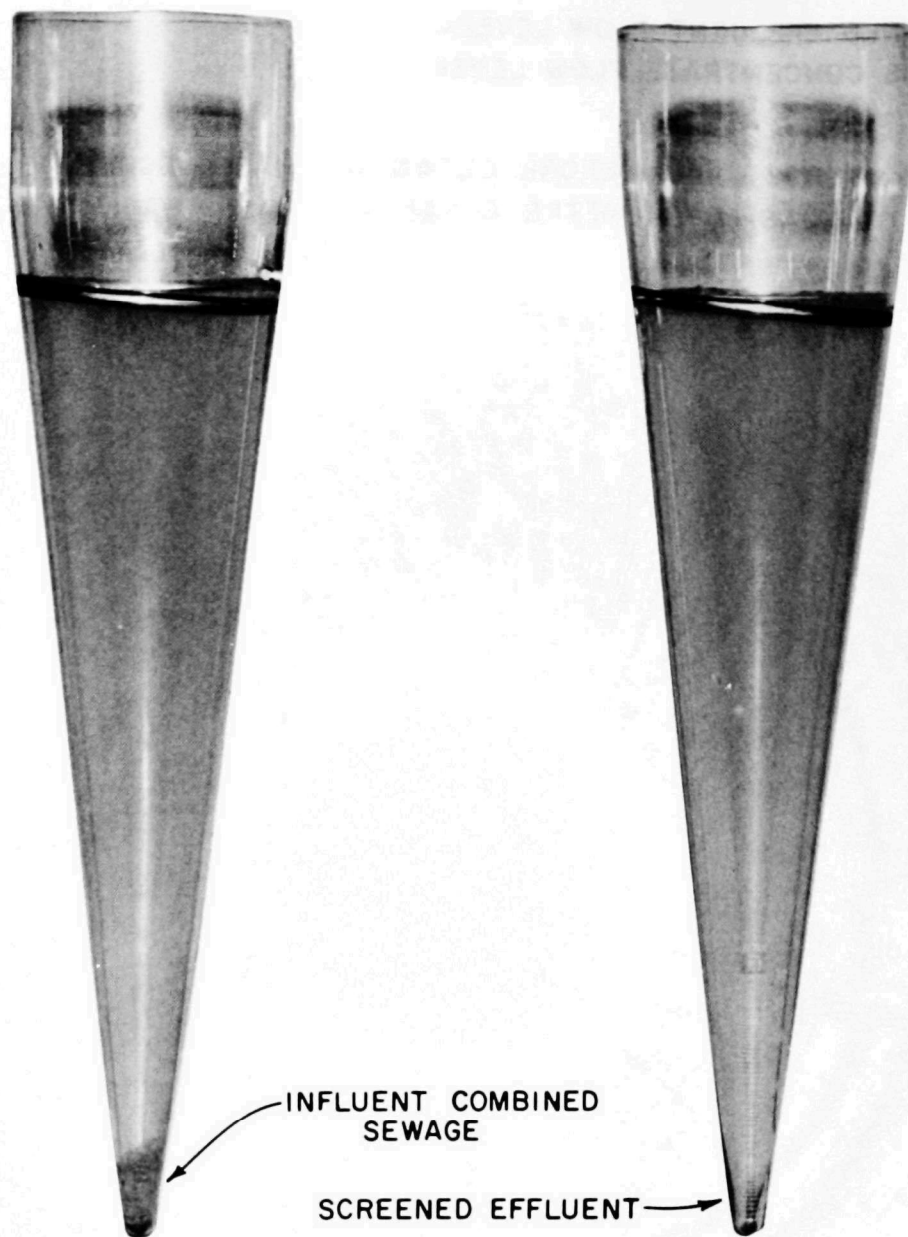


FIGURE 8  
IMHOFF CONE COMPARISON  
OF  
SCREENED AND UNSCREENED FLOWS

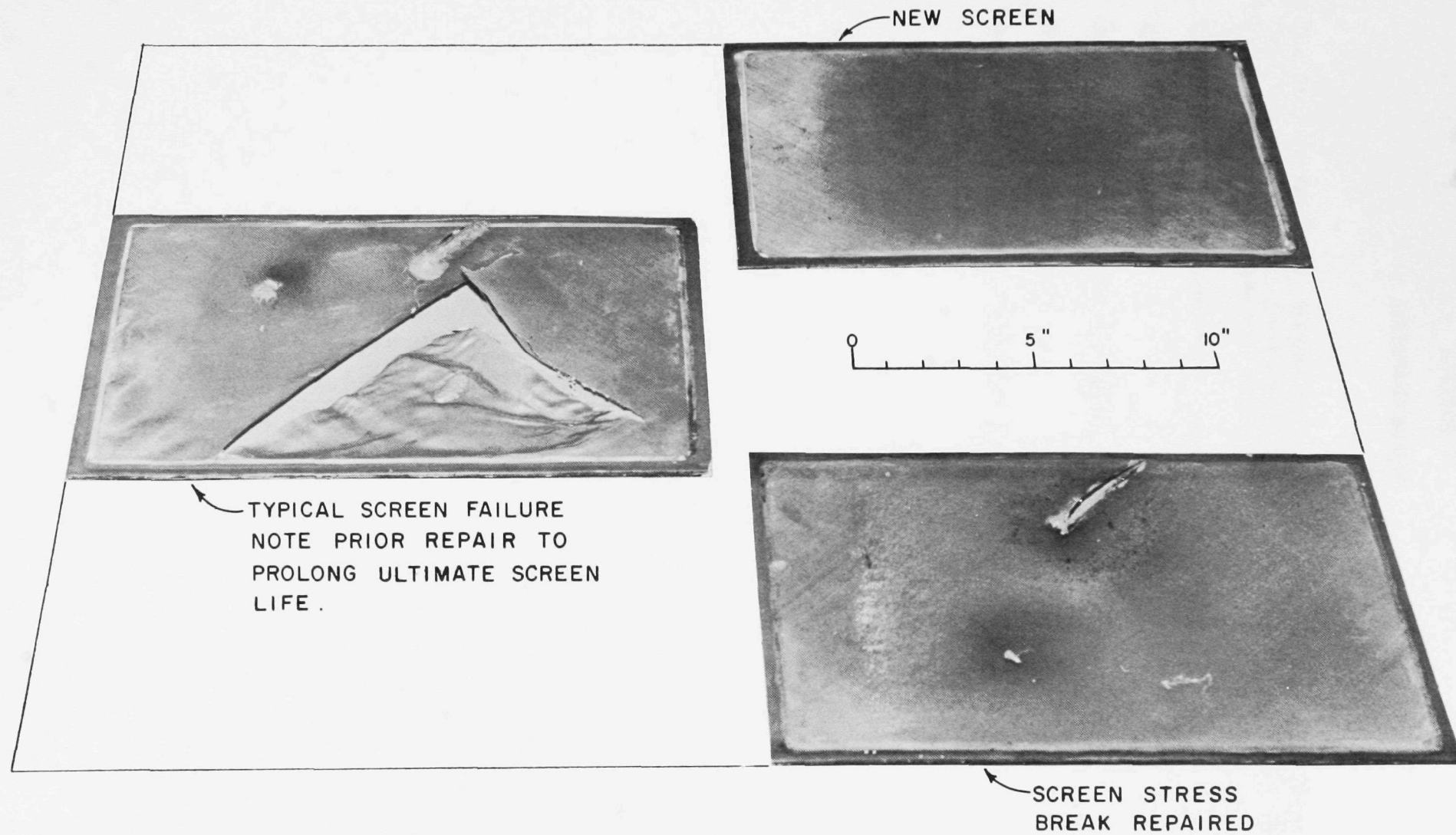


FIGURE 9  
TYPICAL SCREENS

## SECTION 7

### ACKNOWLEDGMENTS

The City of Portland, Oregon acknowledges SWECO, INC. of Los Angeles, California, for their cooperation and assistance in conducting this study for the Environmental Protection Agency.

Donald Hernandez, Project Officer - E.P.A.

City of Portland:

Joseph P. Niehuser, Chief, Bureau of Sanitary Engineering

H. Tim Neketin, Chemist

Harry K. Dennis, Jr., Technical Investigator

## T A B L E S

REMOVAL RATES RAIN CAUSED FLOW

26 OBSERVATIONS

3 MGD

CHARACTERISTIC	INFLUENT			EFFLUENT			PERCENT REMOVAL <sup>1</sup>		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
SETTLEABLE SOLIDS mg/L	59.5	22	180	28.2	2	120	54.8	16.1	92.5
SUSPENDED SOLIDS mg/L	111.8	54	246	81.6	36	192	26.6	6.1	56.4
C O D mg/L	180.5	79	303	152.2	60	281	15.5	5.9	31

1- 26 OBSERVATIONS (SEE APPENDIX)

TABLE 4

REMOVAL RATES DRY-WEATHER FLOW

20 OBSERVATIONS

3 MGD

CHARACTERISTIC	INFLUENT			EFFLUENT			PERCENT REMOVAL <sup>1</sup>		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
SETTLEABLE SOLIDS mg/L	46.4	28	92	17.1	6	44	62.7	12	85.7
SUSPENDED SOLIDS mg/L	85.3	54	128	57.6	38	82	31.9	10.7	45.7
C O D    mg/L	217	67	303	188.8	57	272	13.5	5.3	27.8

<sup>1</sup>- 20 OBSERVATIONS (SEE APPENDIX)

TABLE 5



REMOVAL RATES RAIN CAUSED FLOW

6 OBSERVATIONS

2.5 MGD

CHARACTERISTIC	INFLUENT			EFFLUENT			PERCENT REMOVAL <sup>1</sup>		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
SETTLEABLE SOLIDS mg/L	65	38	176	26.3	0	82	64.2	35	100
SUSPENDED SOLIDS mg/L	111	70	224	77.7	52	136	28.0	20	39.3
C O D mg/L	273.5	248	299	207.5	182	233	224.1	22.1	26.6

1- 6 OBSERVATIONS (SEE APPENDIX)

TABLE 6

REMOVAL RATES DRY-WEATHER FLOW

31 OBSERVATIONS

2.5 MGD

CHARACTERISTIC	INFLUENT			EFFLUENT			PERCENT REMOVAL <sup>1</sup>		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
SETTLEABLE SOLIDS mg/L	68.8	30	182	21.9	6	66	68.8	39.4	93.3
SUSPENDED SOLIDS mg/L	113.8	62	246	71.6	38	134	35.9	20	53.6
C O D    mg/L	284.4	160	405	244.3	123	347	14.7	7.1	23.1

1- 31 OBSERVATIONS (SEE APPENDIX)

TABLE 7

REMOVAL RATES DRY-WEATHER FLOW

20 OBSERVATIONS

3 MGD high velocity

CHARACTERISTIC	INFLUENT			EFFLUENT			PERCENT REMOVAL <sup>1</sup>		
	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.	MEAN	MIN.	MAX.
SETTLEABLE SOLIDS mg/L	74.8	46	134	32.3	8	64	57.5	33.3	82.6
SUSPENDED SOLIDS mg/L	124.1	86	186	88	60	122	28.7	16.1	38.1
C O D mg/L	350	24	532	305	206	502	12.9	5.6	24.7

1- 20 OBSERVATIONS (SEE APPENDIX)

TABLE 8

## SECTION 8

### GLOSSARY

**AVERAGE DAILY DRY WEATHER FLOW** - The flow from a complete sewer system, or a defined portion thereof, measured in total gallons throughout a 24-hour period (expressed in millions of gallons per day).

**BAR SCREEN** - A screen composed of parallel bars, either vertical or inclined, placed in a waterway to catch debris, and from which the screenings may be raked. (Also called a rack).

**BOD<sub>5</sub>** - Five-day Biochemical Oxygen Demand is that amount of oxygen utilized in aerobic decomposition of a waste material during a five-day incubation at constant temperature.

**CLEANING PHASE** - That part of the screening cycle during which the sewage pumps are off and the screens are being automatically washed.

**COD** - Chemical oxygen demand is a measure of the oxygen necessary to stabilize most of the oxidizable compounds in a waste.

**COMBINED SEWER SYSTEM** - A system of sewers receiving both surface runoff and sewage.

**COMPOSITE SAMPLE** - Integrated sample collected by taking a portion at regular time intervals, with sample size varying with flow; or taking uniform portions on a time schedule varying with the total flow.

**CONCENTRATE** - That portion of the flow (solids and liquid) which does not pass through the screens.

**DISSOLVED OXYGEN** - Usually designated as D.O. The oxygen dissolved in sewage or other liquid usually expressed in milligrams per liter or per cent of saturation.

**DISSOLVED SOLIDS** - Solids which are present in solution.

**EFFICIENCY** - The ratio of the actual performance of a device to the theoretically perfect performance sometimes expressed as a percentage.

**EFFLUENT** - Liquid flowing out of a basin or treatment plant.

**EFFLUENT WEIR** - A weir at the outflow end of a sedimentation basin or other hydraulic structure.

**GENERAL SCREEN LIFE** - Collective term used in referring to any or all of ultimate, initial, or service screen lives.

GREASE - In sewage, grease includes fats, waxes, free fatty acids, calcium and magnesium soaps, mineral oils, and other nonfatty materials. Substances soluble in n-hexane.

GRIT - The heavy mineral matter in water or sewage, such as sand, gravel, cinders, etc.

HIGH VELOCITY - The impingement velocity upon the screens when the screen cage is rotating at 65 RPM, the extension piece coupled to the influent line and the deflection plate is closed down over the end of the extension.

HYDRAULIC EFFICIENCY - The ratio of screened effluent flow to influent flow.

INFLUENT - Liquid flowing into a basin or treatment plant.

INITIAL SCREEN LIFE - The number of working hours on a screen until it sustains its first damage, repairable or not.

LOW VELOCITY - The impingement velocity upon the screens when the screen cage is rotating at 55 RPM, the deflection plate is raised and there are no alterations to the influent line.

mg/L - Milligrams per liter.

MGD - Million gallons per day.

OUTFALL SEWER - The outlet or structure through which sewage is finally discharged.

PER CENT CONCENTRATE -  $(100) (\text{Concentrate Flow}) / (\text{Influent flow})$

PRIMARY TREATMENT - The removal of settleable organic and inorganic solids by the process of sedimentation.

SCREENING CYCLE - The sequential events between sewage pump start-up and the conclusion of the cleaning phase.

SCREENING PHASE - That part of the screening cycle during which sewage is being pumped to the screens.

SEDIMENTATION - The process of subsidence and deposition of suspended matter carried by water, sewage, or other liquids, by gravity. It is usually accomplished by reducing the velocity of the liquid below the point where it can transport the suspended material.

SERVICE SCREEN LIFE - The number of working hours on a screen between repairs.

SETTLEABLE SOLIDS - That matter in sewage which will not stay in suspension during the settling period.

SEWAGE TREATMENT PLANT - Man-made structures which subject sewage to treatment by physical, chemical, or biological processes for the purpose of removing or altering its objectionable constituents, and rendering it less offensive or dangerous.

STORM SEWER - A sewer which carries storm water and surface water, street wash and other wash waters or drainage, but excludes sewage and industrial wastes. (Also called a Storm Drain).

SEPARATE SYSTEM - A sewer system comprised exclusively of sanitary sewers which carry only sewage and to which storm water, surface water, and ground water are not intentionally admitted; also referred to as "sanitary system" or separate sanitary system.

SUSPENDED SOLIDS - Non-filterable residue (expressed in parts per million, ppm, or milligrams per liter, mg/L).

TOTAL SOLIDS - The solids in water, sewage, or other liquids.

ULTIMATE SCREEN LIFE - The number of working hours on a screen until it sustains irreparable damage.

VOLATILE SOLIDS - The quantity of solids in water, sewage or other liquid lost on ignition of the total solids.

## SECTION 9

### APPENDICES

	PAGE
SCREEN CLEANING AGENTS - - - - -	57
DATA PRESENTATION - - - - -	59
SUPPLEMENTAL DATA - - - - -	81
DETAILED METHOD OF ANALYSIS - - - - -	83
SCREEN LIFE (High Velocity) - - - - -	85
SCREEN LIFE (Low Velocity) - - - - -	87

## SCREEN CLEANING AGENTS

Early in the program it was found that sodium hypochlorite did not clean the screens adequately and a minor research project ensued. Several screens were taken to the laboratory where they were wetted and various solutions applied to them to determine the best cleaning agent. Concentrated sodium hydroxide, potassium hydroxide, 14% sodium hypochlorite, acetone and chloroform were tried and only chloroform appeared to have an affect on the screen residue. A solvent parts cleaner was tried under actual working conditions; it successfully cleaned the screens but the high water temperature combined with its volatile nature made it unacceptable. A soluble pine oil, Zif, Formula 409, Vestal Eight and a few other similar type cleaners offered limited effectiveness.

A cleaner called Zep 9658 (designed for asphalt and tar removal) was tried and was found to do a very good job cleaning the screens. This may not be the ultimate cleaner as no evaluation of its economic or environmental implications was attempted, but it performed satisfactorily at this installation and no effort was exerted to find a better cleaner.



SCREEN VELOCITY (55 RPM)

RUN NO.		1	2	3	5	6	7	8	10	11	12	13
TIME		0900 1000	1300 1400	0900 1000	1300 1400	0900 1000	1300 1400	1300 1400	0900 1000	1300 1400	2000 2100	0900 1000
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
SULLIVAN FLOW-MGD		24	24	21	30	22	27	30	20	25	22	20
PROCESS STREAM FLOW-MGD		2.8	2.8	2.5	3.0	2.8	2.9	2.9	2.8	3.1	3.0	2.9
HYDRAULIC EFFICIENCY	START	0.754	0.764	0.820	0.808	0.799	0.811	0.834	0.842	0.834	0.860	0.834
	STOP	0.659	0.598	0.570	0.664	0.659	0.665	0.873	0.623	0.653	0.642	0.666
SCREENING PHASE (MINUTES)		7.5	6.1	9.5	4.5	9.5	6.1	6.0	11.5	8.0	6.1	25.0
INFLUENT	SETT. SOLIDS mg/L	60	48	56	50	112	42	64	58	94	50	64
	SUSP. SOLIDS mg/L	88	90	120	84	164	80	132	106	126	116	112
	C O D mg/L				252	405	272	396	347	260		314
	CL <sub>2</sub> REQUIREMENT mg/L								14.6			
EFFLUENT	SETT. SOLIDS mg/L	18	10	28	16	16	24	30	8	34	22	24
	SUSP. SOLIDS mg/L	46	54	88	54	76	64	102	68	76	88	84
	C O D mg/L				211	347	239	368	289	210		272
	CL <sub>2</sub> REQUIREMENT mg/L								15.0			
% REMOVAL SETT. SOLIDS		70	79.2	50	68	85.7	42.8	53.1	86.2	63.8	56.0	62.5
% REMOVAL SUSP. SOLIDS		47.7	40.0	26.7	35.7	53.6	20.0	23.9	35.8	39.7	24.1	25.0
% REMOVAL C O D					16.3	14.3	12.1	7.1	16.7	19.2		13.4

SCREEN VELOCITY (55 RPM)

RUN NO.		16	20	21	22	24	25	26	27	28	29	30
TIME		1300 1400	1300 1400	1700 1800	0800 0900	1300 1400	1700 1800	2000 2100	0900 1000	1300 1400	1700 1800	2000 2100
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
SULLIVAN FLOW-MGD		30	30	32	22	30	28	24	24	24	23	26
PROCESS STREAM FLOW-MGD		3.1	3.0	3.3	3.0	3.0	3.0	2.8	2.7	2.8		2.7
HYDRAULIC EFFICIENCY	START	0.814	0.908	0.850	0.798	0.825	0.817	0.862	0.849	0.862		0.866
	STOP	0.659	0.647	0.745	0.685	0.653	0.643	0.600	0.649	0.637		0.623
SCREENING PHASE (MINUTES)		8.0	5.0	9.5	7.0	14.0	14.0	15.0	20.0	8.0	8.0	8.0
INFLUENT	SETT. SOLIDS mg/L	56	82	114	182	60	66	32	34	50	30	32
	SUSP. SOLIDS mg/L	80	124	160	246	92	140	64	82	80	78	62
	C O D mg/L	194									272	
	CL <sub>2</sub> REQUIREMENT mg/L									12.2	15.6	
EFFLUENT	SETT. SOLIDS mg/L	20	22	44	34	30	40	4	8	22	8	8
	SUSP. SOLIDS mg/L	54	70	94	114	48	108	38	58	54	62	48
	C O D mg/L	156								223		
	CL <sub>2</sub> REQUIREMENT mg/L								11.8	13.0		
% REMOVAL SETT. SOLIDS		64.3	73.2	61.4	81.3	50.0	39.4	87.5	76.5	56	73.3	75.0
% REMOVAL SUSP. SOLIDS		32.5	43.5	41.2	53.6	47.8	22.8	40.6	29.3	32.5	20.5	22.6
% REMOVAL C O D		19.6								18.0		

SCREEN VELOCITY (55 RPM)

RUN NO.		31	32	33	34	35	36	37	39	40		
TIME		0800 0900	1300 1400	1700 1800	2000 2100	0800 0900	1700 1800	2000 2100	1300 1400	1700 1800		
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry		
SULLIVAN FLOW-MGD		20	26	21	20	20	21	25	21	20		
PROCESS STREAM FLOW-MGD		2.7	3.0	3.1	2.8	2.8	3.0	3.0	3.1	2.9		
HYDRAULIC EFFICIENCY	START	0.863	0.860	0.852	0.853	0.853	0.834	0.873	0.843	0.834		
	STOP	0.642	0.679	0.679	0.604	0.693	0.686	0.700	0.698	0.666		
SCREENING PHASE (MINUTES)		14.5	6.1	13.7	7.0	11.5	5.5	5.5	5.5	12.0		
INFLUENT	SETT. SOLIDS mg/L	120	56	46	54	38	70	64	82	112		
	SUSP. SOLIDS mg/L	144	100	90	114	68	126	112	122	184		
	C O D mg/L	160	309			157			360			
	CL <sub>2</sub> REQUIREMENT mg/L	12.3	12.0			12.3			12.7			
EFFLUENT	SETT. SOLIDS mg/L	8	8	6	16	8	26	10	32	52		
	SUSP. SOLIDS mg/L	40	58	54	78	48	88	60	78	134		
	C O D mg/L	123	264			144			330			
	CL <sub>2</sub> REQUIREMENT mg/L	10.4	12.0			11.3			12.0			
% REMOVAL SETT. SOLIDS		93.3	85.7	87	70.4	78.9	62.8	84.4	61.0	53.6		
% REMOVAL SUSP. SOLIDS		72.2	42	40.0	31.6	29.4	30.2	46.4	36.1	27.2		
% REMOVAL C O D		23.1	14.6			8.3			8.3			

SCREEN VELOCITY (55 RPM)

RUN NO.		104	105	129	160	162	169	170	171	173	174	175
TIME		0900 1000	1300 1400	0900 1000	0100 0200	1300 1400	1700 1800	2000 2100	0100 0200	0900 1000	1300 1400	1630 1730
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
SULLIVAN FLOW-MGD		25	20	22	21	31	23	22	20	25	30	21
PROCESS STREAM FLOW-MGD		3.5	3.3	3.5	3.3	3.4	3.3	3.3	3.3	3.3	3.3	3.3
HYDRAULIC EFFICIENCY	START	0.860	0.840	0.890	0.884	0.850	0.901	0.886	0.901	0.839	0.870	0.882
	STOP	0.694	0.660	0.726	0.826	0.872	0.880	0.729	0.753	0.794	0.760	0.660
SCREENING PHASE (MINUTES)		15.0	10.6	5.5	20.0	4.0	10.0	3.5	8.0	5.0	4.0	12.0
INFLUENT	SETT. SOLIDS mg/L	40	50	36	86	34	40	44	28	52	42	42
	SUSP. SOLIDS mg/L	86	106	78	128	74	84	84	68	98	98	94
	C O D mg/L	288	280	266	127	245	249	245	162	252	298	303
	CL <sub>2</sub> REQUIREMENT mg/L	13.3	12.7	13.0	10.0	11.3	6.9	8.7	8.7	10.0	9.7	9.4
EFFLUENT	SETT. SOLIDS mg/L	16	20	12	28	10	14	12	16	18	8	12
	SUSP. SOLIDS mg/L	64	82	52	76	52	56	54	48	68	66	66
	C O D mg/L	228	232	223	100	232	205	205	148	222	267	272
	CL <sub>2</sub> REQUIREMENT mg/L	12.0	12.7	12	11.1	13.1	7.6	8.0	9.0	8.4	8.4	9.0
% REMOVAL SETT. SOLIDS		60	60.0	66.7	67.4	70.6	65	72.7	42.8	65.4	81	71.4
% REMOVAL SUSP. SOLIDS		25.5	22.6	33.3	40.6	29.7	33.3	35.7	29.4	30.6	32.6	29.8
% REMOVAL C O D		20.8	17.1	16.2	21.2	5.3	17.7	16.3	8.6	11.9	10.4	10.2

SCREEN VELOCITY (55 RPM)

RUN NO.	176	181	196	199	205	206	210	211	215		
TIME	2000 2100	0100 0200	0100 0200	1300 1400	2000 2100	0100 0200	1700 1800	2000 2100	1300 1400		
WEATHER	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry		
SULLIVAN FLOW-MGD	20	21	12	20	19	12	20	19	30		
PROCESS STREAM FLOW-MGD	3.3	3.3	3.3	3.3	3.3	3.4	3.4	3.3	3.2		
HYDRAULIC EFFICIENCY	0.853 0.656	0.915 0.900	0.900 0.866	0.876 0.746	0.874 0.660	0.902 0.757	0.886 0.744	0.882 0.706	0.862 0.732		
START STOP											
SCREENING PHASE (MINUTES)	10.0	20.0	25.0	7.5	15.0	10.0	23.0	10.0	5.0		
INFLUENT	SETT. SOLIDS mg/L	52	32	50	42	32	92	40	52	42	
	SUSP. SOLIDS mg/L	94	56	76	74	64	120	78	76	70	
	C O D mg/L	232	79	67	243	170	152	234	217	232	
	CL <sub>2</sub> REQUIREMENT mg/L	7.6	12.9	12.4	12.4	11.1	6.6	38.3	9.6	9.7	
EFFLUENT	SETT. SOLIDS mg/L	14	26	44	6	8	34	14	20	10	
	SUSP. SOLIDS mg/L	58	50	64	54	38	68	52	46	38	
	C O D mg/L	219	57	60	234	133	135	213	184	208	
	CL <sub>2</sub> REQUIREMENT mg/L	8.7	16.2	13.1	11.0	8.3	9.0	34.8	11.3	9.3	
% REMOVAL SETT. SOLIDS		73.1	18.8	12.0	85.7	75	63.0	65	61.5	76.2	
% REMOVAL SUSP. SOLIDS		38.3	10.7	15.8	27.0	40.6	43.3	33.3	39.5	45.7	
% REMOVAL C O D		5.6	27.8	10.4	3.7	21.8	11.2	9	15.2	10.3	

SCREEN VELOCITY (55 RPM)

RUN NO.		9	14	15	17	18	38					
TIME		0900 1000	1700 1800	2000 2100	1700 1800	2000 2100	0800 0900					
WEATHER		Rain	Rain	Rain	Rain	Rain	Rain					
SULLIVAN FLOW-MGD		30	32	30	33	32	30					
PROCESS STREAM FLOW-MGD		3.1	3.1	2.6	3.0	2.8	3.1					
HYDRAULIC EFFICIENCY	START	0.824	0.840	0.843	0.808	0.804	0.814					
	STOP	0.688	0.681	0.617	0.647	0.634	0.751					
SCREENING PHASE (MINUTES)		8.0	9.5	5.0	7.0	8.0	20.0					
INFLUENT	SETT. SOLIDS mg/L	54	38	40	38	44	176					
	SUSP. SOLIDS mg/L	104	90	100	70	78	224					
	C O D mg/L	248					299					
	CL <sub>2</sub> REQUIREMENT mg/L						13					
EFFLUENT	SETT. SOLIDS mg/L	30	0	26	6	14	82					
	SUSP. SOLIDS mg/L	82	62	80	54	52	136					
	C O D mg/L	182					233					
	CL <sub>2</sub> REQUIREMENT mg/L						14.3					
% REMOVAL SETT. SOLIDS		44.4	100	35.0	84.2	68.2	53.4					
% REMOVAL SUSP. SOLIDS		21.2	31.1	20.0	22.8	33.3	39.3					
% REMOVAL C O D		26.6					22.1					

## SCREEN VELOCITY (55 RPM)

RUN NO		124	125	130	131	162	168	177	178	179	180	183
TIME		0900 1000	1900 2000	1700 1800	2000 2100	0900 1000	1300 1400	0900 1000	1300 1400	1630 1730	2000 2100	0900 1000
WEATHER		Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain
SULLIVAN FLOW-MGD		55	55	55	45	45	33	48	58	44	58	38
PROCESS STREAM FLOW-MGD		3.3	3.2	3.3	3.6	3.3	3.3	3.3	3.3	3.4	3.3	3.3
HYDRAULIC START EFFICIENCY STOP		0.870 0.650	0.910 0.800	0.878 0.732	0.907 0.838	0.862 0.748	0.860 0.746	0.859 0.726	0.875 0.837	0.904 0.787	0.900 0.724	0.884 0.837
SCREENING PHASE (MINUTES)		40.0	10.5	16.5	12.0	10.5	4.5	10.0	15.0	10.0	20.0	0.5
INFLUENT	SETT SOLIDS mg/L	66	28	54	56	52	56	130	42	50	58	42
	SUSP SOLIDS mg/L	114	58	94	92	92	122	224	98	120	128	70
	C O D mg/L	228	172	211	116	162	303	237	158	169	193	206
	CL <sub>2</sub> REQUIREMENT mg/L	12.7	11.6	10.3	8.1	12.1	11.1	10.4	14.1	13.8	11.1	12.9
EFFLUENT	SETT. SOLIDS mg/L	42	12	20	28	26	20	60	22	20	22	14
	SUSP. SOLIDS mg/L	88	46	66	68	68	88	162	66	86	98	44
	C O D mg/L	176	120	159	99	144	281	175	109	151	175	179
	CL <sub>2</sub> REQUIREMENT mg/L	13.1	14.0	10.8	7.1	11.1	12.4	10.0	14.1	13.1	11.1	11.7
% REMOVAL SETT. SOLIDS		36.4	57.1	63.0	50	50	64.3	53.8	47.6	60	62.1	66.7
% REMOVAL SUSP. SOLIDS		22.8	20.7	29.8	26.1	26.1	27.9	27.7	32.6	28.3	23.7	37.1
% REMOVAL C O D		22.8	30.2	24.6	14.6	11.1	7.3	26.2	31.0	10.6	9.3	13.1

SCREEN VELOCITY (55 RPM)

RUN NO.		184	185	186	190	191	192	193	194	195	198	202
TIME		1300 1400	0100 0200	1000 1100	2000 2100	0100 0200	0900 1000	1300 1400	1700 1800	2000 2100	0900 1000	0900 1000
WEATHER		Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain	Rain
SULLIVAN FLOW-MGD		38	54	38	36	48	30	50	44	40	33	34
PROCESS STREAM FLOW-MGD		3.3	3.3	3.3	3.5	3.3	3.3	3.3	3.3	3.5	3.3	3.3
HYDRAULIC EFFICIENCY		0.857 0.755	0.866 0.804	0.837 0.750	0.899 0.742	0.878 0.797	0.850 0.731	0.884 0.739	0.901 0.696	0.904 0.810	0.875 0.790	0.868 0.794
SCREENING PHASE (MINUTES)		14.0	12.0	4.0	16.5	5.5	6.5	8.0	56.0	35.0	15.0	5.0
INFLUENT	SETT. SOLIDS mg/L	180	22	80	56	62	32	50	82	38	34	40.
	SUSP. SOLIDS mg/L	246	54	124	134	98	84	88	148	84	84	98
	C O D mg/L	180	79	80	122	191	187	172	155	140	148	215
	CL <sub>2</sub> REQUIREMENT mg/L	14.8	10.4	9.4	10.7	9.7	9.7	13.1	9.7	13.5	17.6	11.1
EFFLUENT	SETT. SOLIDS mg/L	120	2	6	14	52	12	36	36	16	12	18
	SUSP. SOLIDS mg/L	192	36	54	94	92	62	70	100	62	68	72
	C O D mg/L	162	70	226	109	148	176	121	129	114	138	178
	CL <sub>2</sub> REQUIREMENT mg/L	12.9	9.6	9.4	11.1	10.4	9.4	11.4	10.4	14.5	11.7	10.0
% REMOVAL SETT. SOLIDS		33.3	90.9	92.5	75	16.1	62.5	28.0	56.1	57.9	64.7	55
% REMOVAL SUSP. SOLIDS		22.0	33.3	56.4	29.8	6.1	26.2	20.4	32.4	26.2	19	26.5
% REMOVAL C O D		10.0	11.4	8.9	10.8	22.5	5.9	29.6	16.8	18.6	6.8	17.2



SCREEN VELOCITY (55 RPM)

RUN NO.		203	204	208	209						
TIME		1300 1400	1700 1800	0900 1000	1300 1400						
WEATHER		Rain	Rain	Rain	Rain						
SULLIVAN FLOW-MGD		33	28	58	30						
PROCESS STREAM FLOW-MGD		3.2	3.3	3.3	3.3						
HYDRAULIC EFFICIENCY	START	0.844	0.878	0.917	0.900						
	STOP	0.686	0.703	0.894	0.836						
SCREENING PHASE (MINUTES)		4.2	20.0	15.0	8.0						
INFLUENT	SETT. SOLIDS mg/L	64	96	42	34						
	SUSP. SOLIDS mg/L	116	198	72	68						
	C O D mg/L	239	248	70	136						
	CL <sub>2</sub> REQUIREMENT mg/L	10.4	14.2	12.1	12.1						
EFFLUENT	SETT. SOLIDS mg/L	36	38	34	14						
	SUSP. SOLIDS mg/L	88	138	64	50						
	C O D mg/L	208	226	60	125						
	CL <sub>2</sub> REQUIREMENT mg/L	8.7	9.7	12.1	12.1						
% REMOVAL SETT. SOLIDS		43.8	60.4	19	58.8						
% REMOVAL SUSP. SOLIDS		24.1	29.6	11.1	26.5						
% REMOVAL C O D		13	8.9	14.3	8.1						

SCREEN VELOCITY (65 RPM)

RUN NO		1-1	1-2	1-3	1-4	1-5	1-6	1-7	1-8	1-11	1-12	1-13
TIME		0900 1000	1300 1400	0900 1000	1300 1400	0900 100	0900 1000	1300 1400	0900 1000	0900 1000	1300 1400	0900 1000
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
SULLIVAN FLOW-MGD			30	25	25	24	24	30	20	32	25	21
PROCESS STREAM FLOW-MGD		3.8	3.4	3.5	3.5	3.3	3.6	3.3	3.3	3.6	3.6	3.3
HYDRAULIC EFFICIENCY	START	0.848	0.832	0.846	0.855	0.858	0.806	0.866	0.892	0.836	0.856	0.865
	STOP	0.785	0.774	0.780	0.786	0.793	0.782	0.799	0.740	0.761	0.761	0.740
SCREENING PHASE (MINUTES)		6.0	3.7	12.8	5.5	4.5	2.8	14.5	25.0	3.3	4.5	13.0
INFLUENT	SETT. SOLIDS mg/L		76	46	75	54	48	108	90	52	118	50
	SUSP. SOLIDS mg/L	125	148	105	149	112	98	162	154	86	162	116
	C O D mg/L	246	310	356	301	339	267	281	532	303	377	476
	CL <sub>2</sub> REQUIREMENT mg/L											
EFFLUENT	SETT. SOLIDS mg/L		26	8	29	36	16	58	48	24	64	22
	SUSP. SOLIDS mg/L	99	110	72	119	94	68	114	122	60	116	88
	C O D mg/L	224	265	322	275	318	234	250	502	269	328	410
	CL <sub>2</sub> REQUIREMENT mg/L											
% REMOVAL SETT. SOLIDS			65.8	82.6	61.3	33.3	66.6	46.3	46.6	53.8	45.8	56
% REMOVAL SUSP. SOLIDS		20.8	25.7	31.4	20.1	16.1	30.6	29.6	20.8	30.2	28.4	24.1
% REMOVAL C O D		8.9	14.5	9.6	8.6	6.2	12.4	11.0	5.6	11.2	13.0	13.9

SCREEN VELOCITY (65 RPM)

RUN NO.		1-14	1-15	1-16	1-17	1-18	1-19	2-2	2-3	2-4		
TIME		1300 1400	0900 1000	1300 1400	0900 1000	1300 1400	2100 2200	1700 1800	1700 1800	2100 2200		
WEATHER		Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry		
SULLIVAN FLOW-MGD		22	30	30	22	30	30	21	25	20		
PROCESS STREAM FLOW-MGD		3.3	3.4	3.4	3.5	3.3	3.6	3.3	3.3	3.1		
HYDRAULIC EFFICIENCY	START	0.850	0.841	0.841	0.832	0.858	0.838	0.884	0.858	0.805		
	STOP	0.758	0.760	0.733	0.811	0.768	0.745	0.791	0.756	0.754		
SCREENING PHASE (MINUTES)		5.0	10	8.1	11.5	7.0	4.5	11.5	3.5	3.5		
INFLUENT	SETT. SOLIDS mg/L	70	72	134	60	54	92	92	54	68		
	SUSP. SOLIDS mg/L	118	96	186	106	100	138	122	94	96		
	C O D mg/L	348	336	352	422	341	302	246	504	254		
	CL <sub>2</sub> REQUIREMENT mg/L											
EFFLUENT	SETT. SOLIDS mg/L	30	38	60	18	28	38	28	16	28		
	SUSP. SOLIDS mg/L	84	66	116	72	78	90	76	60	62		
	C O D mg/L	302	253	291	327	315	263	211	448	206		
	CL <sub>2</sub> REQUIREMENT mg/L											
% REMOVAL SETT. SOLIDS		57.1	47.2	55.2	70	48	58.7	69.6	70.4	58.8		
% REMOVAL SUSP. SOLIDS		28.8	31.2	37.6	32.1	22.0	34.8	37.7	36.2	35.4		
% REMOVAL C O D		13.2	24.7	17.3	22.5	7.6	12.9	14.2	11.1	18.8		

SUPPLEMENTAL DATA

1970 DATE		7-27	8-3	8-10	9-21	9-28	10-6	10-13	10-20	11-19	11-24	11-30
INFLUENT	5 Day BOD mg/L	145	175	180	145	130	110		100	90	90	85
	Hexane Solubles mg/L	43	32	16	18	12.8	23	32	23	64	37.6	
	Total Solids mg/L	570	435	343	437	373	331	356	245	273	382	294
EFFLUENT	5 Day BOD mg/L	165	180	30	145	115	120		85	105	75	70
	Hexane Solubles mg/L	29	35	8	11	10.4	16	24	30	36.4	32.4	24
	Total Solids mg/L	444	383	315	377	274	310	325	224	275	358	274
CONCENTRATE	5 Day BOD mg/L	190	250	105	185	205	195			115	120	110
	Hexane Solubles mg/L	47	43	18	13	16	28	32		34.4	42.8	28
	Total Solids mg/L	728	607	482	489	508	533	470		326	480	409

## DETAILED METHOD OF ANALYSIS

### SETTLEABLE SOLIDS

1. Determine suspended solids on a well-mixed sample.
2. Pour approximately three quarts of well mixed sample into a wide-mouthed plastic gallon jug. Depth should be at least 20 cm, diameter at least 9 cm.
3. Affix a glass siphon tube to a sturdy holder with the end of the tube in the middle of the jug and allow to remain quiescent for one hour. After one hour siphon off 250 ml being careful not to disturb the material.
4. Determine suspended solids on the 250 ml portion. Express result as mg/L non-settleable matter.
5. Calculation:  
$$\text{mg/L sett. matter} = \text{mg/L susp. matter} - \text{mg/L non-sett. matter}$$

### CHLORINE REQUIREMENT

A rapid method of determining the amount of chlorine which must be applied to produce a measurable chlorine residual will be used. Contact time is arbitrarily set at fifteen minutes. Standardized dilute sodium hypochlorite (Clorox) is added to the sample in sufficient quantity to produce a measurable residual after the contact period. The residual chlorine level is titrated with standard sodium thiosulfate/starch-iodide, and the chlorine requirement is calculated as the difference between the amount applied and the residual.

### GREASE

Filterable n-hexane soluble substance will be determined by mixing 250 ml of sample and 250 ml of n-hexane in a liter separatory funnel, filtering the hexane phase through Whatman's #40 and weighing the residue upon evaporation of the filtrate at 85°C.

SCREEN NUMBER	INITIAL SCREEN LIFE-HRS.	NUMBER OF REPAIRS	ULTIMATE SCREEN LIFE-HRS.
101	Non-Reparable	--	19.
102	Non-Reparable	--	32.25
103	Non-Reparable	--	36.
104	23.5	7	40.
105	Non-Reparable	--	19.5
106	25.25	6	95.25
107	35.5	6	65.5
108	Non-Reparable	--	16.5
109	Non-Reparable	--	18.5
111	15.5	2	19.5
112	16.	1	23.
113	1.75	5	31.25
114	28.	1	30.
115	15.25	3	30.25
117	22.	3	30.75
118	12.5	5	32.75
120	23.75	2	44.25
122	26.5	1	33.5

SCREEN LIFE 3.1 - 3.8 MGD - HIGH VELOCITY 65 RPM

SCREEN NUMBER	INITIAL SCREEN LIFE-HRS.	NUMBER OF REPAIRS	ULTIMATE SCREEN LIFE-HRS.
152	49.25	3	395.75
153	336.75	2	516.
154	226.5	10	428.25
155	228.75	5	397.25
156	216.	3	320.5
157	Non-Reparable	--	239.25
158	32.	7	325.75
159	118.	1	190.5
160	299.	4	364.
161	216	4	324.
162	232.25	3	355.
163	283.25	3	306.5
164	288.5	2	315.25
165	316.75	1	321.75
166	312.5	4	385.5
167	293.	4	385.25
168	283.25	3	345.25

SCREEN LIFE - 2.5-3.6 MGD - LOW VELOCITY 55 RPM

1	Accession Number	2	Subject Field & Group	<b>SELECTED WATER RESOURCES ABSTRACTS</b> INPUT TRANSACTION FORM
			Ø 5 D	

5	Organization
	Bureau of Sanitary Engineering, City of Portland, Oregon

6	Title
	DEMONSTRATION OF ROTARY SCREENING FOR COMBINED SEWER OVERFLOWS,

10	Author(s)	16	Project Designation
	Neketin, Tim H.		EPA Contract No. 14-12-128 Modification No. 7
	Dennis, Harry K., Jr.	21	Note

22	Citation
----	----------

23	Descriptors (Starred First)
	storm runoff, water pollution control

25	Identifiers (Starred First)
	combined sewage treatment *, high-rate screening, solids removal efficiency, C.O.D. removal efficiency

27	Abstract
	The objective of this demonstration was to determine screen durability, solids removal, COD removal, and hydraulic efficiency of rotary fine screening of storm-caused combined sewer overflows.

2300 gpm were evenly distributed to a 60 inch diameter rotating (55 rpm) screen cage holding 18 ft<sup>2</sup> of 165 mesh stainless steel screens (105 micron opening, 47.1 percent open area). During a screening cycle a concentrate sensor stopped the sewage pumps, ending the screening phase and initiating a 30 second cleaning phase during which the screens were automatically washed. At the end of the cleaning phase the pumps restarted automatically and a new cycle began.

Performance on storm-caused combined sewage flow averaged 54.8 percent removal of settleable solids, 26.6 percent removal of suspended solids, and 15.5 percent removal of COD. Duration of the screening phases averaged 14.6 minutes with average hydraulic efficiencies dropping from 0.880 to 0.668.

The ultimate screen life varied from a minimum of 190.5 hours to a maximum of 516 hours with an average of 346. Screens required an average of 3.5 repairs during this life. (Schmidt - Portland)

Abstractor	Institution
------------	-------------



Continued from inside front cover....

11022 --- 08/67	Phase I - Feasibility of a Periodic Flushing System for Combined Sewer Cleaning
11023 --- 09/67	Demonstrate Feasibility of the Use of Ultrasonic Filtration in Treating the Overflows from Combined and/or Storm Sewers
11020 --- 12/67	Problems of Combined Sewer Facilities and Overflows, 1967 (WP-20-11)
11023 --- 05/68	Feasibility of a Stabilization-Retention Basin in Lake Erie at Cleveland, Ohio
11031 --- 08/68	The Beneficial Use of Storm Water
11030 DNS 01/69	Water Pollution Aspects of Urban Runoff, (WP-20-15)
11020 DIH 06/69	Improved Sealants for Infiltration Control, (WP-20-18)
11020 DES 06/69	Selected Urban Storm Water Runoff Abstracts, (WP-20-21)
11020 --- 06/69	Sewer Infiltration Reduction by Zone Pumping, (DAST-9)
11020 EXV 07/69	Strainer/Filter Treatment of Combined Sewer Overflows, (WP-20-16)
11020 DIG 08/69	Polymers for Sewer Flow Control, (WP-20-22)
11023 DPI 08/69	Rapid-Flow Filter for Sewer Overflows
11020 DGZ 10/69	Design of a Combined Sewer Fluidic Regulator, (DAST-13)
11020 EKO 10/69	Combined Sewer Separation Using Pressure Sewers, (ORD-4)
11020 --- 10/69	Crazed Resin Filtration of Combined Sewer Overflows, (DAST-4)
11024 FKN 11/69	Stream Pollution and Abatement from Combined Sewer Overflows - Bucyrus, Ohio, (DAST-32)
11020 DWF 12/69	Control of Pollution by Underwater Storage
11000 --- 01/70	Storm and Combined Sewer Demonstration Projects - January 1970
11020 FKI 01/70	Dissolved Air Flotation Treatment of Combined Sewer Overflows, (WP-20-17)
11024 DOK 02/70	Proposed Combined Sewer Control by Electrode Potential
11023 FDD 03/70	Rotary Vibratory Fine Screening of Combined Sewer Overflows, (DAST-5)
11024 DMS 05/70	Engineering Investigation of Sewer Overflow Problem - Roanoke, Virginia
11023 EVO 06/70	Microstraining and Disinfection of Combined Sewer Overflows
11024 --- 06/70	Combined Sewer Overflow Abatement Technology
11034 FKL 07/70	Storm Water Pollution from Urban Land Activity
11022 DMU 07/70	Combined Sewer Regulator Overflow Facilities
11024 EJC 07/70	Selected Urban Storm Water Abstracts, July 1968 - June 1970
11020 --- 08/70	Combined Sewer Overflow Seminar Papers
11022 DMU 08/70	Combined Sewer Regulation and Management - A Manual of Practice
11023 --- 08/70	Retention Basin Control of Combined Sewer Overflows
11023 FIX 08/70	Conceptual Engineering Report - Kingman Lake Project
11024 EXF 08/70	Combined Sewer Overflow Abatement Alternatives - Washington, D.C.